

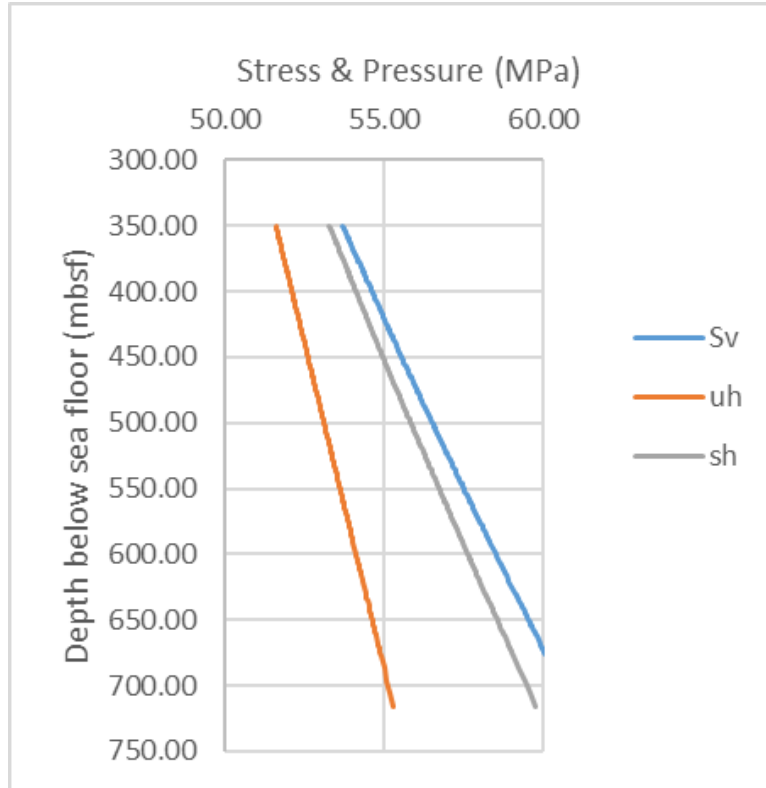
HW-3A: COMPRESSION BEHAVIOR IN THE LOWER SHIKOKU OF THE NANKAI FOLD AND THRUST BELT

ANSWERS

We will study the compression behavior in the Nankai Trough at IODP Site 1173. Figures 1 and 2 provide a general overview of this location.

- 1) See the spreadsheet associated with this homework. For Site 1173, please calculate the overburden from the seafloor downwards. Note that the data start at 350 meters below seafloor. I have provided the total vertical stress (σ_v) and the hydrostatic pressure at 350 mbsf (meters below seafloor) in the spread sheet. The water depth is 4795.7 meters. Assume that the pore pressure is hydrostatic.
 - a. Construct a pressure-depth plot from the seafloor to total depth.
 - i. Include hydrostatic pressure. Assume a water density of 1023 kg/m³.
 - ii. Include the overburden stress. Calculate the overburden stress from 350 meters downward by integrating the bulk density, which you infer from the measured porosity (given in the spread sheet). Assume a solid sediment density of 2750 kg/m³ (this is slightly denser than quartz, but that is because clays are denser than quartz).
 - iii. Include the least principal stress assuming uniaxial strain and that K_0 is 0.8 and that the pore pressure is hydrostatic.

In excel, I calculated density and void ratio. I integrated the density to get the overburden stress. I calculated the hydrostatic pressure. I used the stress ratio equation ($K_0 = \frac{\sigma'_3}{\sigma'_1}$) presented in class to determine the least principal stress (the horizontal total stress)



- 2) Two common expressions used to describe the compression behavior of mudrocks are the 'geotechnical' approach (Eq. 1) and the 'Hubbert' Approach (Eq.2):

$$e_1 = e_0 - C_c \log \left(\frac{\sigma'_1}{\sigma'_0} \right) \quad \text{Eq. 1}$$

$$n = n_0 e^{-\beta \sigma'_v} \quad \text{Eq. 2}$$

$$\sigma'_v = \sigma_v - u \quad \text{Eq. 3}$$

$$e = \frac{n}{1-n} \quad \text{Eq. 4}$$

e_0 is a reference void ratio commonly taken at a vertical effective stress, σ'_0 , of unity, and C_c is a compression coefficient. In Eq. 2, n is porosity, n_0 is a reference porosity at an effective stress of zero and β is a compressibility parameter. Equation 3 is the effective stress equation. In locations where the pore pressure is hydrostatic, u is the hydrostatic pressure u_h .

- Plot void ratio (e) vs. the log of hydrostatic effective stress ($\sigma'_{vh} = \sigma_v - u_h$). Use regression to determine the parameters e_0 , and C_c . After you have derived e_0 , and C_c , then please substitute them in Eq. 1 and plot this line.
- Plot porosity (n) vs. hydrostatic effective stress (σ'_{vh}) and use regression to determine the parameters n_0 , β . After you have derived n_0 & β , then please substitute them in Eq. 2 and plot this line.
- Please determine the value of m_v at 3.0 and 4.0 MPa. Remember that the coefficient of compressibility is the slope of the void ratio vs. effective stress plot divided by $1 + \text{void ratio}$ at the location the slope is taken:

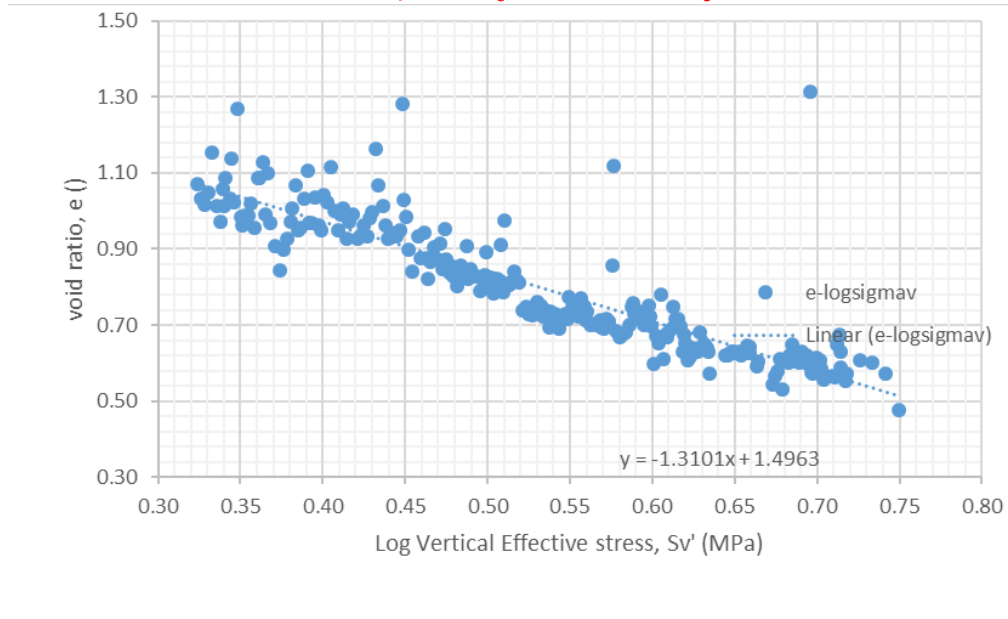
$$m_v = \left(\frac{-1}{1+e} \right) \frac{de}{d\sigma'} \quad \text{Eq. 5}$$

d. Plot porosity vs. depth for the 1173 well adjacent to your pressure vs. depth plot (#1 above).

Step 1 is to determine the compression parameters for the 1173 well using the geotechnical approach. In the figure below, I have performed a linear regression of the following equation:

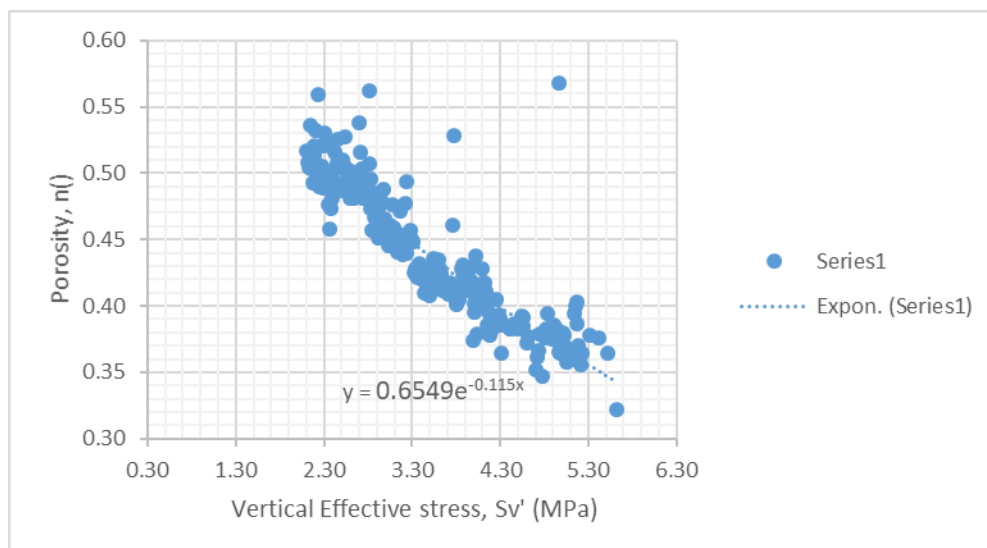
$$e = e_o + C_c \log(\sigma'_v)$$

we find: $e_o = 1.50$, and $C_c = 1.31$



I have repeated the exercise here for the Hubbert approach:

$$n = n_o e^{-\beta \sigma'_v}$$



I find:

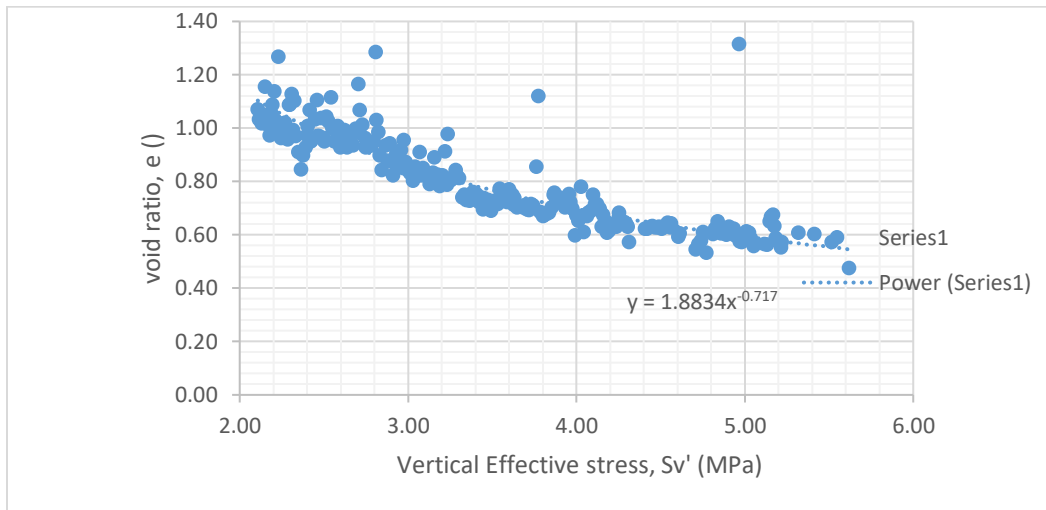
$$n_o = 0.65$$

$$\beta = -0.115 \text{ MPa}^{-1}$$

I leave it to you to plot the porosity vs. depth

m_v is defined in Eq. 5:

A plot of void ratio vs effective stress is:



I fit a power-law function to the plot of void ratio vs. effective stress. In the attached page, I calculate m_v for the stress at 3.0 and 4.0 MPa. I find $m_v = 0.11 \text{ MPa}^{-1}$ at 3 MPa and 0.07 MPa^{-1} at 4 MPa. This demonstrates that m_v decreases with increasing stress. This implies the rock gets stiffer as it compacts. This makes sense because one can envision more particle contacts with decreasing porosity.

I have fit a power law to the plot of void ratio to effective stress.

I find:

$$e = 1.88(\sigma_v')^{-0.717}$$

(Eq. #1)

I use Equation #1 to derive e at 3 and 4 MPa:

$$e_{3\text{MPa}} = 0.85$$

$$e_{4\text{MPa}} = 0.69$$

$$m_v = \frac{1}{1+e} \frac{-de}{d\sigma_v'}$$

(Eq. #2)

To determine $\frac{de}{d\sigma_v'}$, I take the derivative of Eqn #1:

$$e = a\sigma_v'^b$$

$$de = ab\sigma_v'^{(b-1)} d\sigma_v'$$

$$\frac{de}{d\sigma_v'} = ab\sigma_v'^{(b-1)}$$

(Eq. #3)

$$\begin{aligned} \text{Eq \#3: @ 3 MPa } \frac{de}{d\sigma_v'} &= 1.88 \cdot (-.717) (3\text{MPa})^{(-0.717-1)} = -0.204 \\ \text{Eq \#3 @ 4 MPa } & \text{ " " (4 MPa) " " } = -0.12 \end{aligned}$$

$$\text{Eq \#2 } m_v @ 3\text{MPa} = \left[\frac{1}{1+.85} \right] [.204] = \boxed{0.11 \text{ MPa}^{-1}}$$

$$\text{Eq \#2 } m_v @ 4\text{MPa} = \left[\frac{1}{1+.69} \right] [.12] = \boxed{0.07 \text{ MPa}^{-1}}$$