



Particle Filtering for Hydraulic Models: Probabilistic Urban Inundation Modeling and Assimilation-based H-Q Relationships

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Probabilistic Inundation Modeling using Urban Flood Model and PF

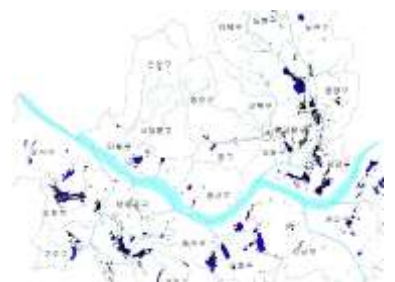
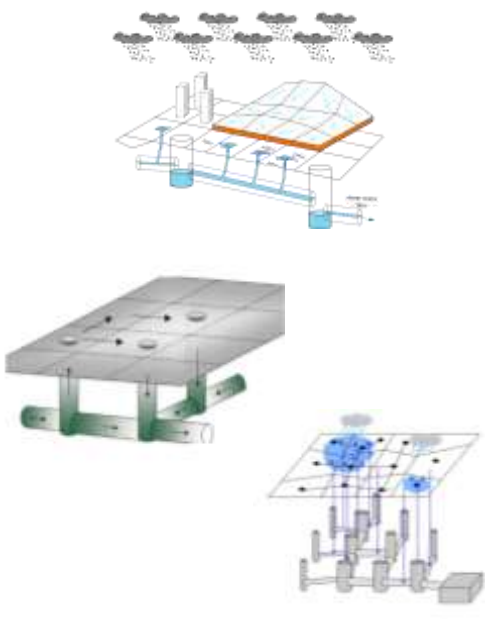
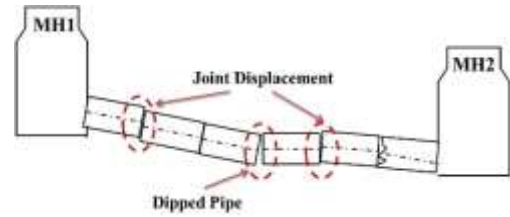
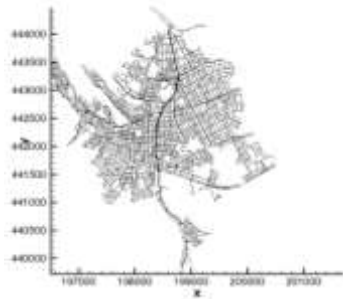
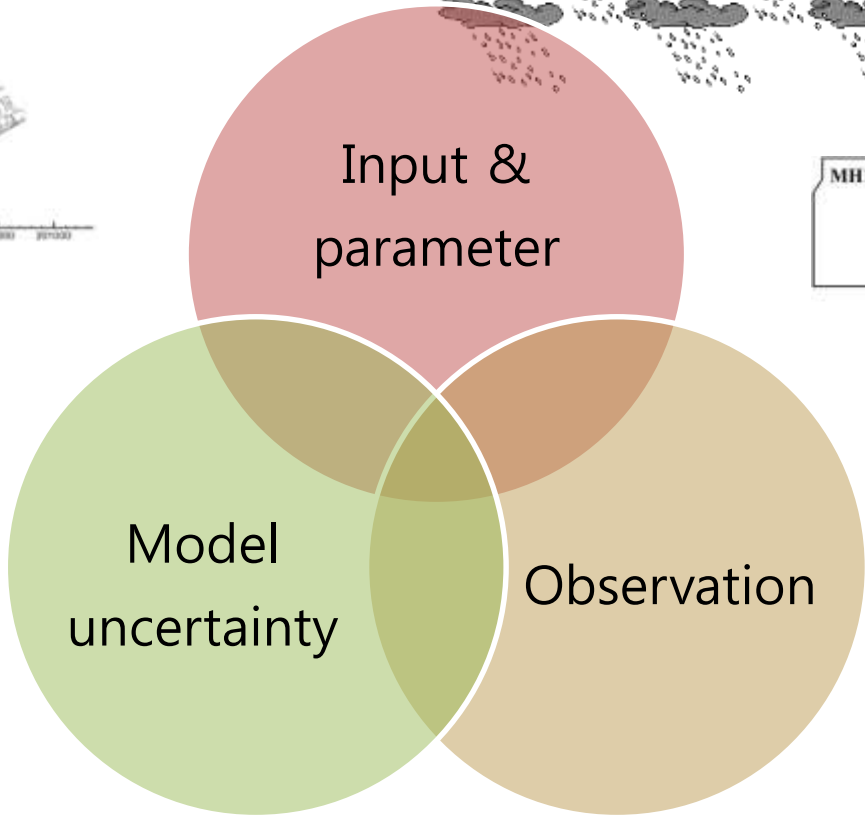


Introduction – Probabilistic urban flood model

Model ensembles

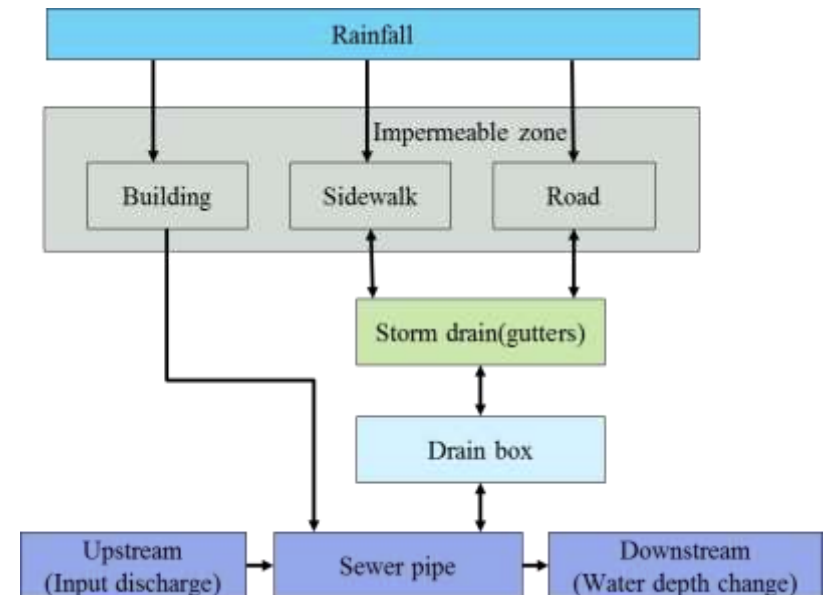
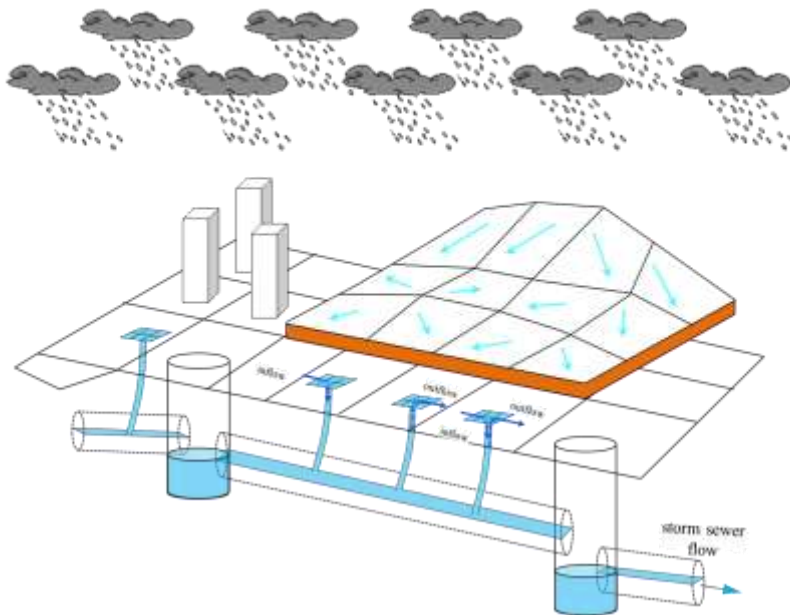


Data assimilation



Integrated urban flood model (DPRI, Kyoto Univ.)

- 2-D inundation model on the ground surface
- 1-D network model of sewer pipes
- Combined by a sub-model to exchange storm water between the ground surface and the sewerage system



□ Governing equations

➤ Ground surface

$$\frac{\partial h}{\partial t} + \frac{\partial M}{\partial x} + \frac{\partial N}{\partial y} = r_e - q_{drain} - q_{sew}$$

$$\frac{\partial M}{\partial t} + \frac{\partial(uM)}{\partial x} + \frac{\partial(vM)}{\partial y} = -gh \frac{\partial H}{\partial x} - \frac{gn^2 M \sqrt{u^2 + v^2}}{h^{4/3}}$$

$$\frac{\partial N}{\partial t} + \frac{\partial(uN)}{\partial x} + \frac{\partial(vN)}{\partial y} = -gh \frac{\partial H}{\partial y} - \frac{gn^2 N \sqrt{u^2 + v^2}}{h^{4/3}}$$

➤ drain box

$$\frac{\partial A}{\partial t} + \frac{\partial Q}{\partial x} = q_{drain}$$

※ FDM & leap-frog methods are adopted

h : water depth

H : water elevation

u : x-direction water velocity

v : y-direction water velocity

M : uh (x-direction flux)

N : vh (y-direction flux)

r_e : effective rainfall

q_{drain} : unit area drainage discharge between ground and drain box

q_{sew} : unit area drainage discharge between ground and sewer pipe

g : acceleration of gravity

n : Manning's roughness coefficient

A : box bottom area

Q : discharge

q_{drain} : unit area drainage discharge between ground and drain box

□ Governing equations (sewer pipe)

$$\frac{\partial A}{\partial t} + \frac{\partial Q}{\partial x} = q_{sew} + q_{drain}$$

$$\frac{\partial Q}{\partial t} + \frac{\partial(uQ)}{\partial x} = -gA \frac{\partial H_p}{\partial x} - gn^2 \frac{|Q|Q}{R^{4/3}A}$$

$$h = \begin{cases} f(A) & : A \leq A_0 \\ D + \frac{(A - A_0)}{b_s} & : A > A_0 \end{cases}$$

$$\phi = 2 \cos^{-1} \left(1 - 2 \frac{h}{d} \right)$$

$$\frac{A}{A_0} = \frac{\phi - \sin \phi}{2\pi}$$

$$\frac{R}{R_0} = 1 - \frac{\sin \phi}{\phi}$$

$$B_s = \frac{gA_0}{a^2}$$

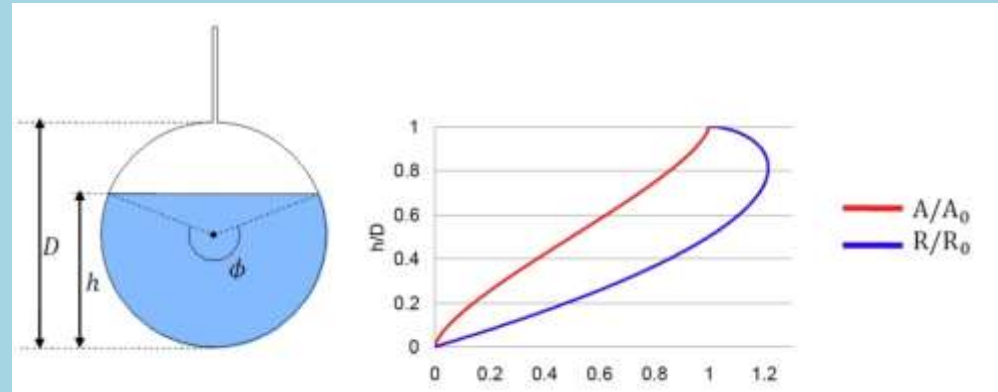


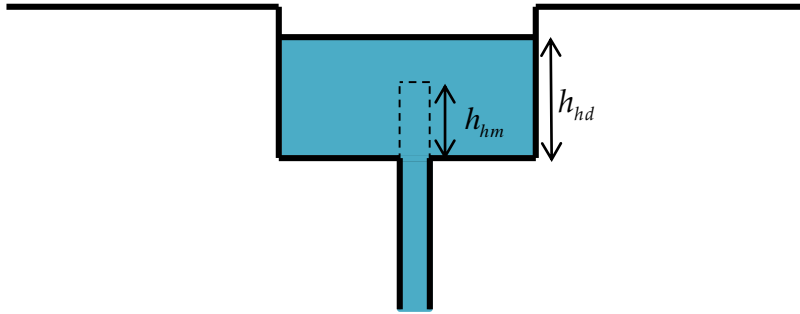
Fig. Hydraulic characteristic curves of the circular sewer pipe

The a is decided as 5.0 m/s in this study

- R : hydraulic radius
- q_{sew} : unit area drainage discharge between ground and sewer pipe
- q_{drain} : unit area drainage discharge between sewer pipe and drain box
- g : acceleration of gravity
- n : Manning's roughness coefficient
- A : stream cross-section
- Q : discharge
- u : water velocity
- H_p : piezometric head
- h : water depth
- B_s : head losses

Interaction model (surface water - sewer pipe)

① Inlet



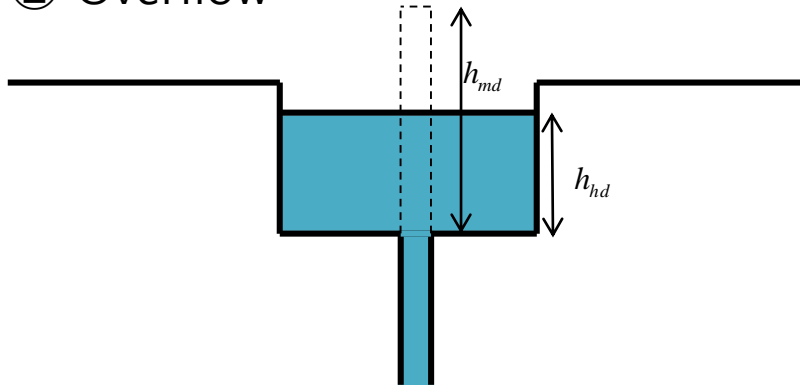
$$\text{if, } h_{hd} - h_{hm} / b_o \geq 0.5$$

$$\text{if, } h_{hd} - h_{hm} / b_o < 0.5$$

$$Q = C_{do} \times A \times \sqrt{2g(h_{hd} - h_{hm})}$$

$$Q = C_{dw} \times L \times \frac{2}{3} \sqrt{2g} (h_{hd} - h_{hm})^{\frac{3}{2}}$$

② Overflow



$$\text{if, } h_{md} - h_{hd} / b_o \geq 0.5$$

$$\text{if, } h_{md} - h_{hd} / b_o < 0.5$$

$$Q = -C_{do} \times A \times \sqrt{2g(h_{md} - h_{hd})}$$

$$Q = -C_{dw} \times L \times \frac{2}{3} \sqrt{2g} (h_{md} - h_{hd})^{\frac{3}{2}}$$

C_{do} : orifice coefficient (0.57)

C_{dw} : weir coefficient (0.48)

Q : discharge

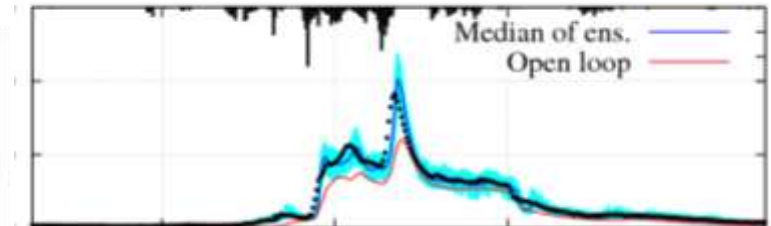
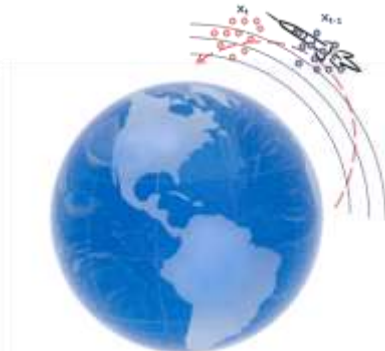
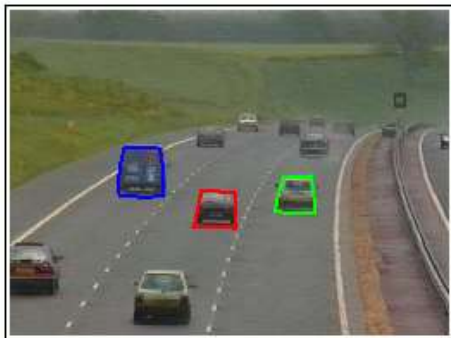
h_{hd} : piezometric head of drain channel

h_{md} : piezometric head of sewer pipe

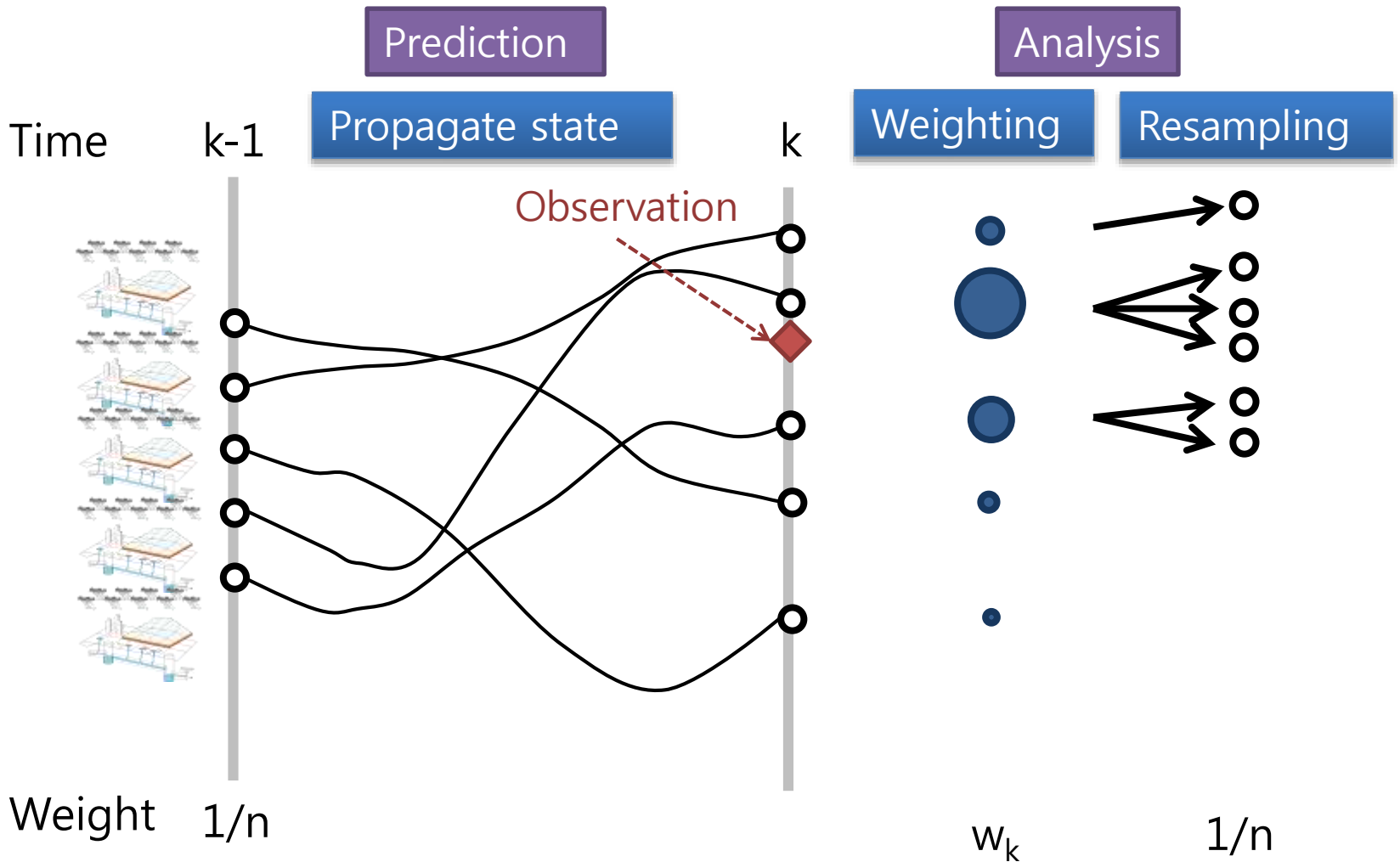
A : area of bottom hole

Particle filtering

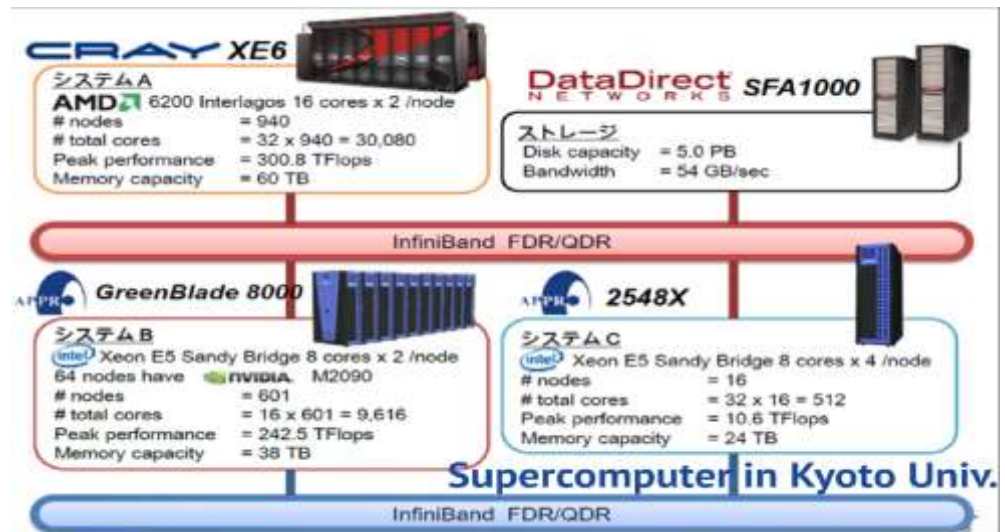
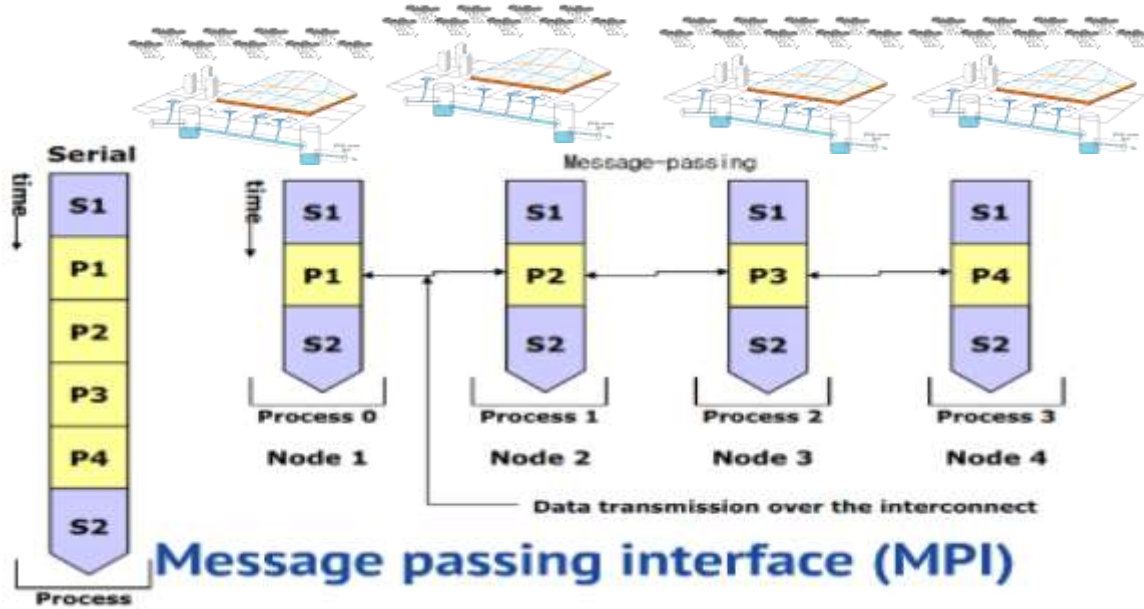
- Also known as sequential Monte Carlo (SMC)
- Applicable for non-linear, non-Gaussian state-space models (most of hydraulic and hydrologic models)
- Point mass (“particle”) representations of probability densities with associated weights
- Expensive computation but easy for parallelization
- Wide-spread applications including image processing, target tracking, and flood forecasting
- Noh, S. J., Rakovec, O., Weerts, A. H., and Tachikawa, Y.: On noise specification in data assimilation schemes for improved flood forecasting using distributed hydrological models. J. Hydrol. in press, 2014.



SIR particle filter

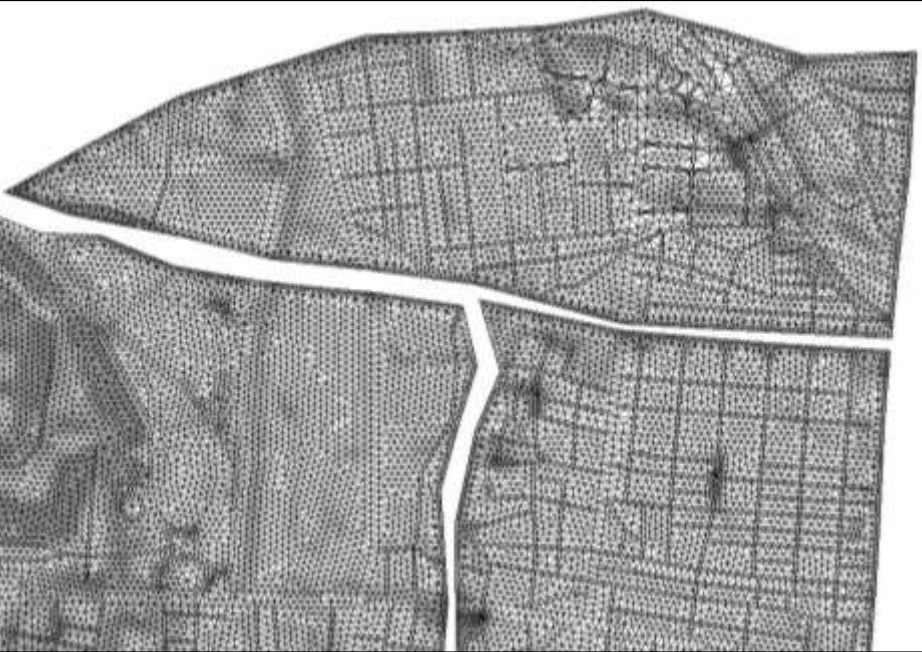
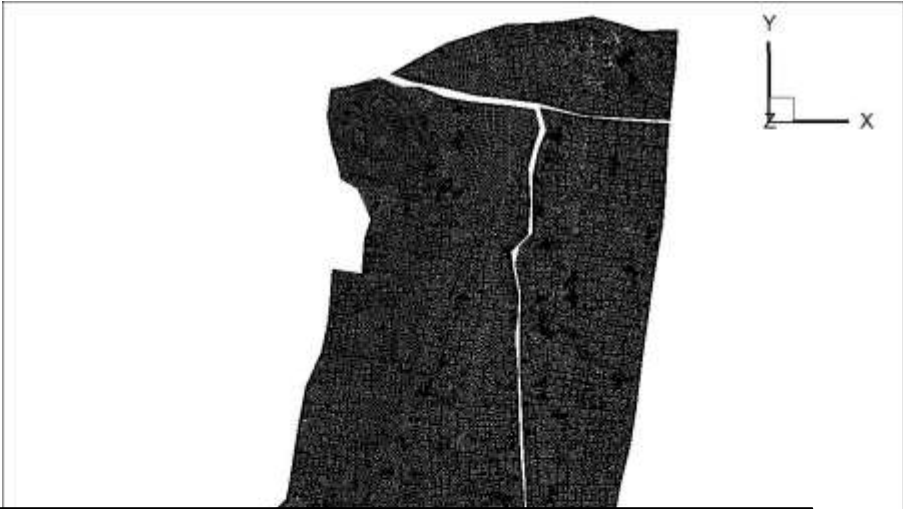


Methodology – Parallel computing

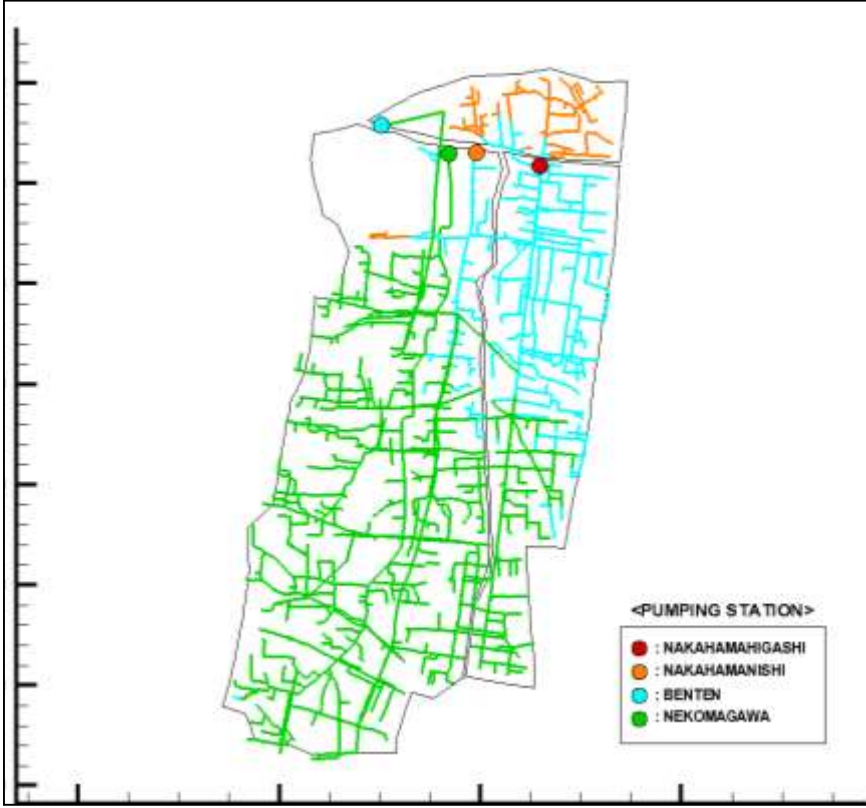


Application – Study area

Urban surface

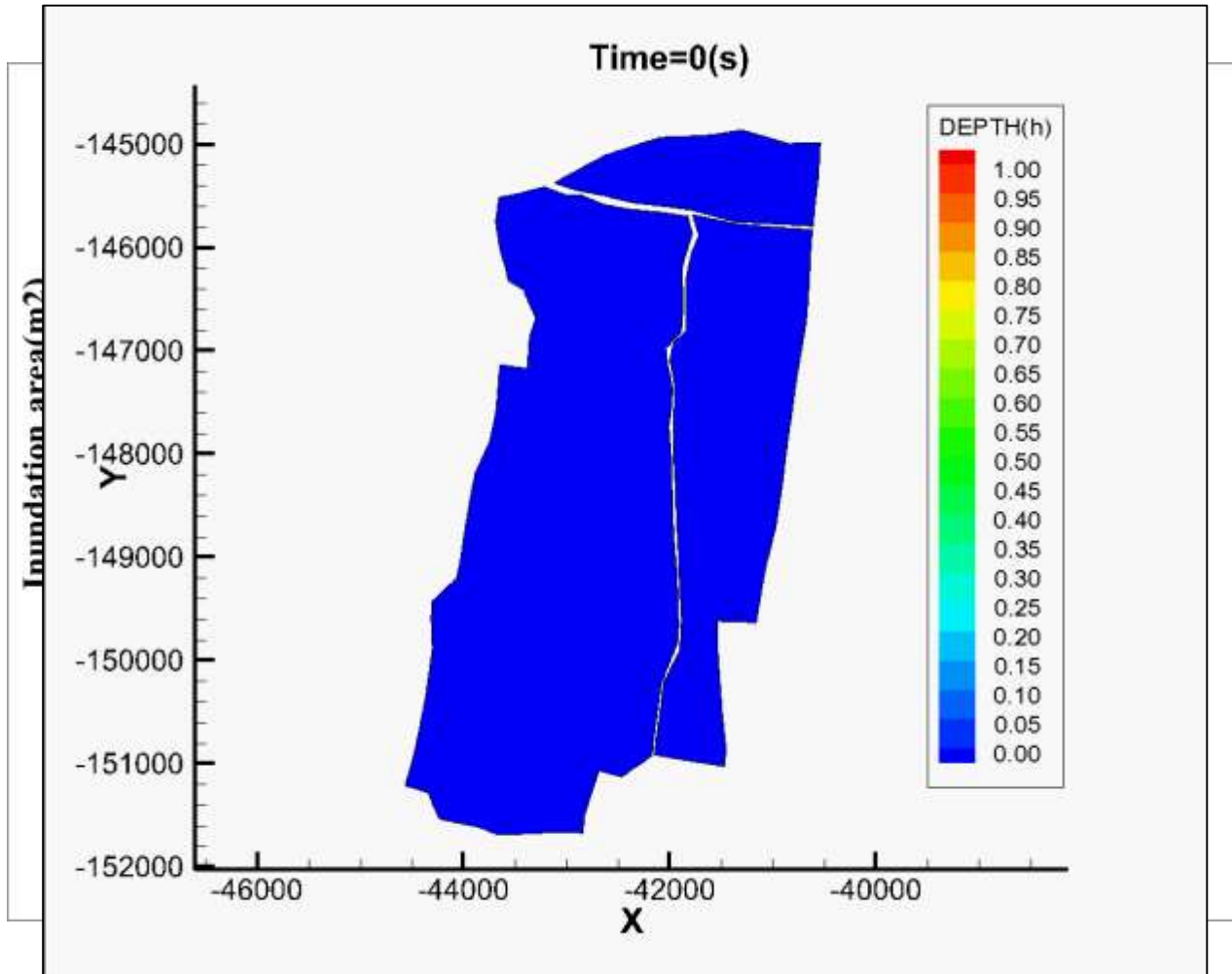


Sewer pipe network



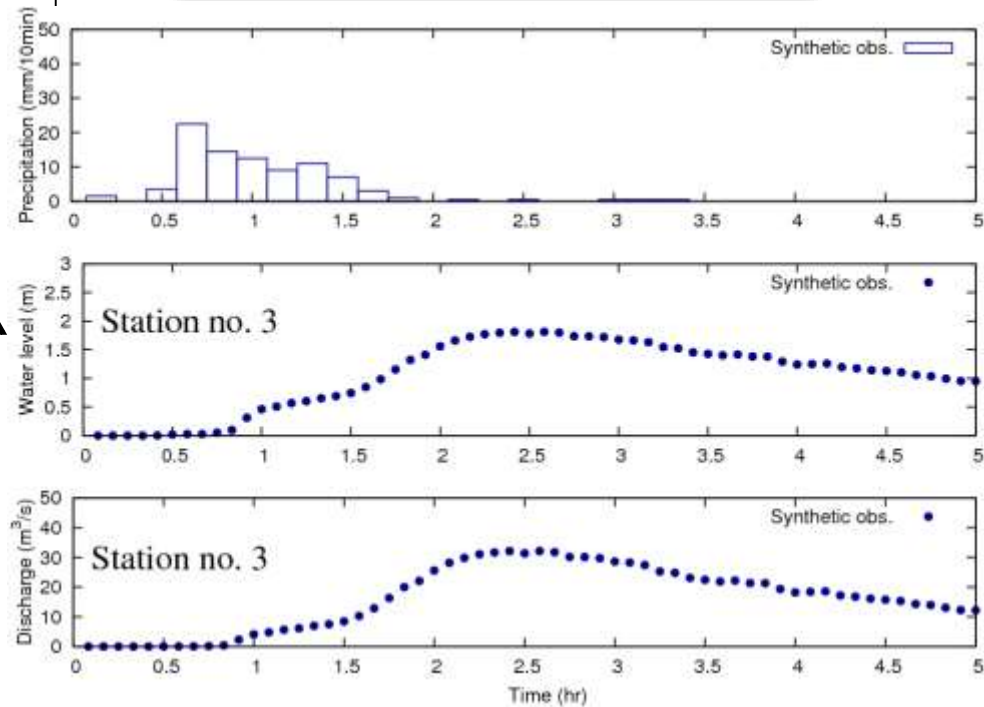
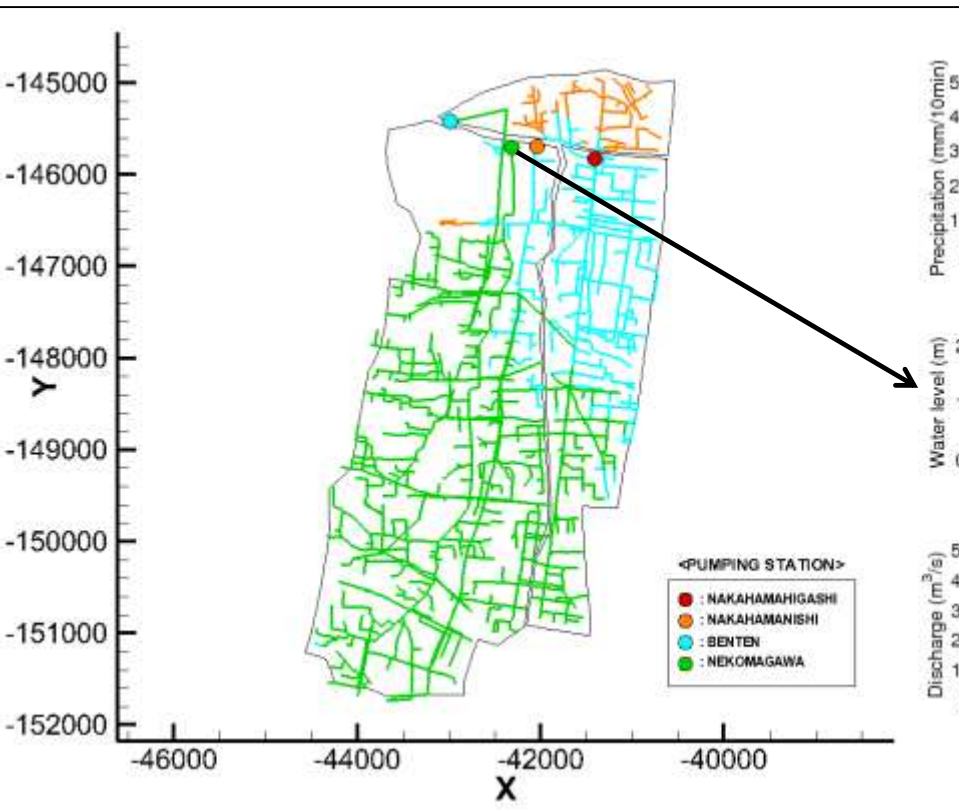
Node : 60,411
Mesh : 117,435
Link : 177,843

Deterministic modeling case



Application – Setup of synthetic experiment

Synthetic observation

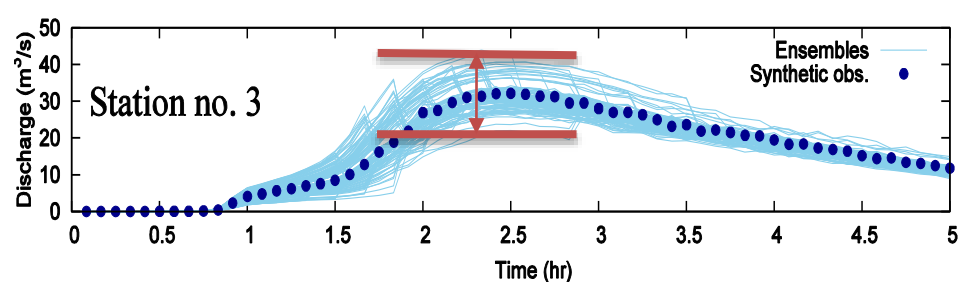
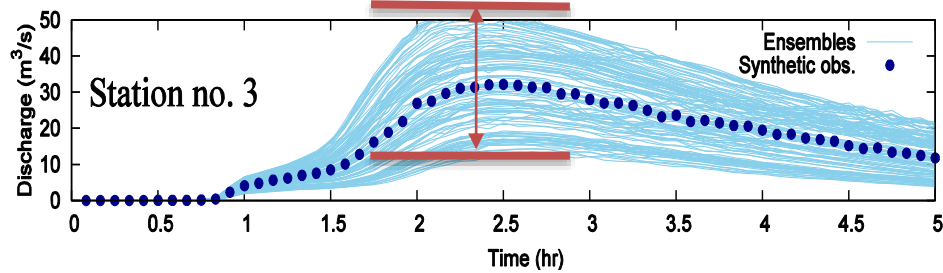
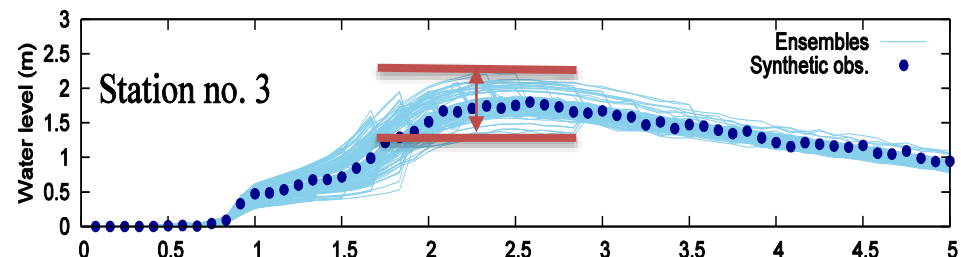
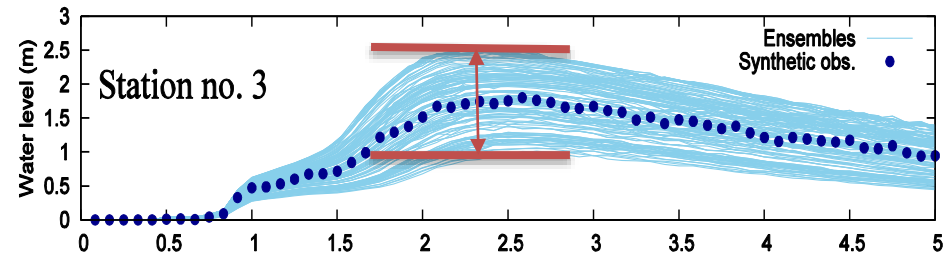
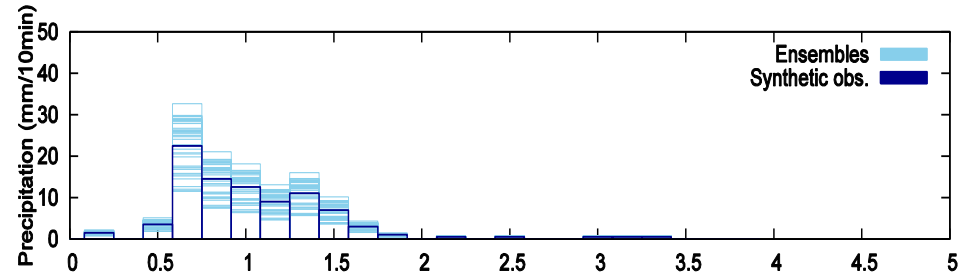
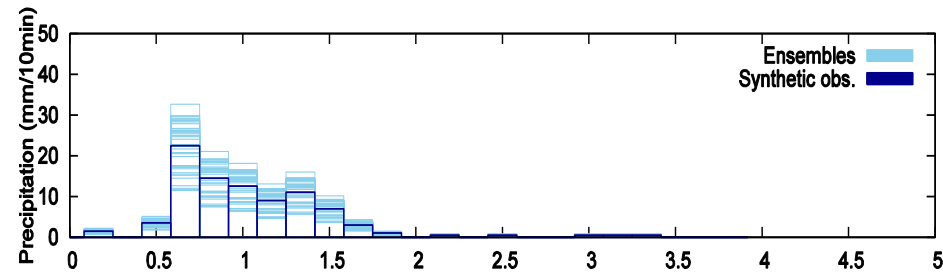


- Water level and discharge at pumping station no. 3 were used as synthetic observation
- Spatial distribution of rainfall was not considered

Application – Comparison on H & Q

Without particle filtering

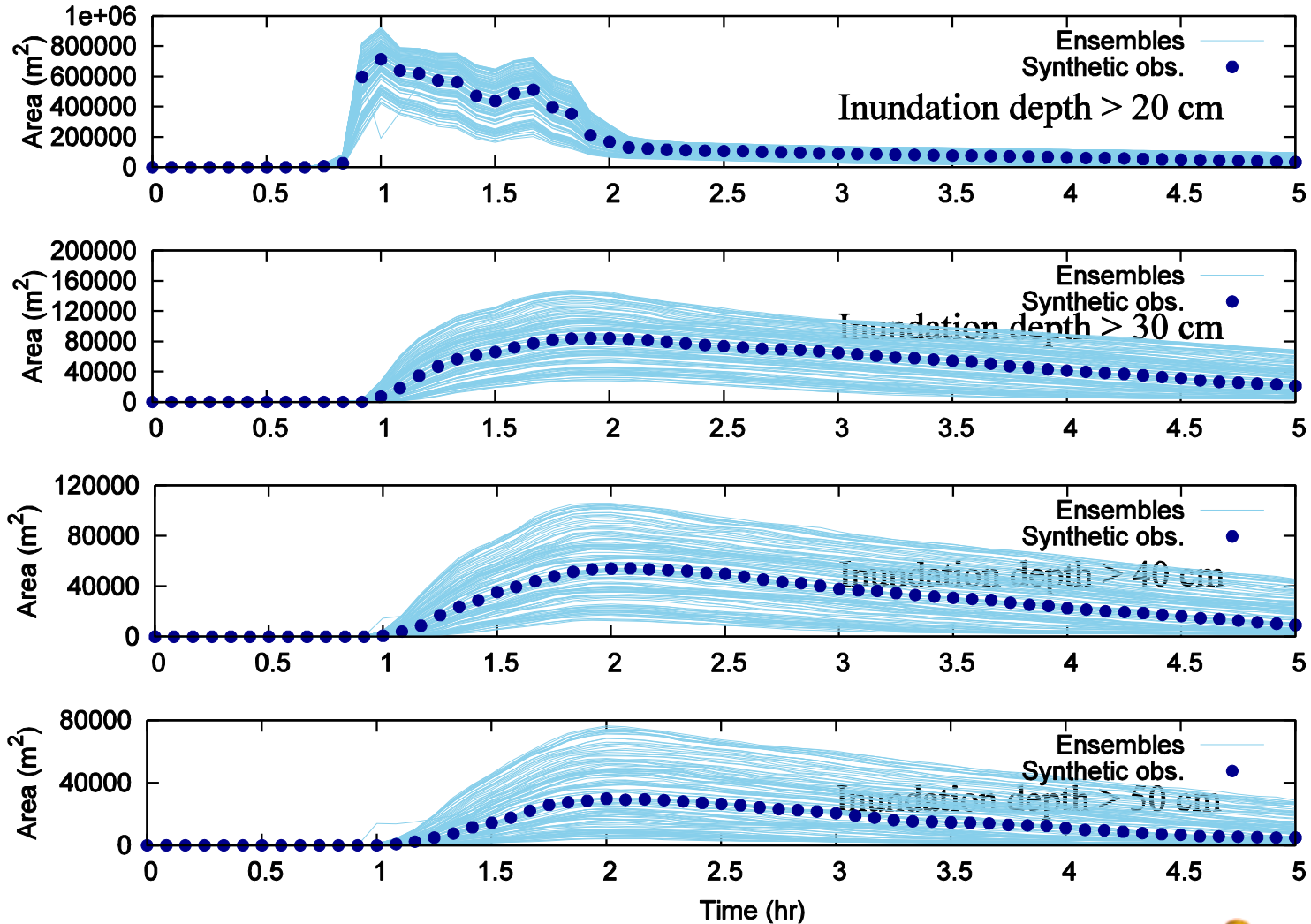
With particle filtering





Application – Comparison on inundation area

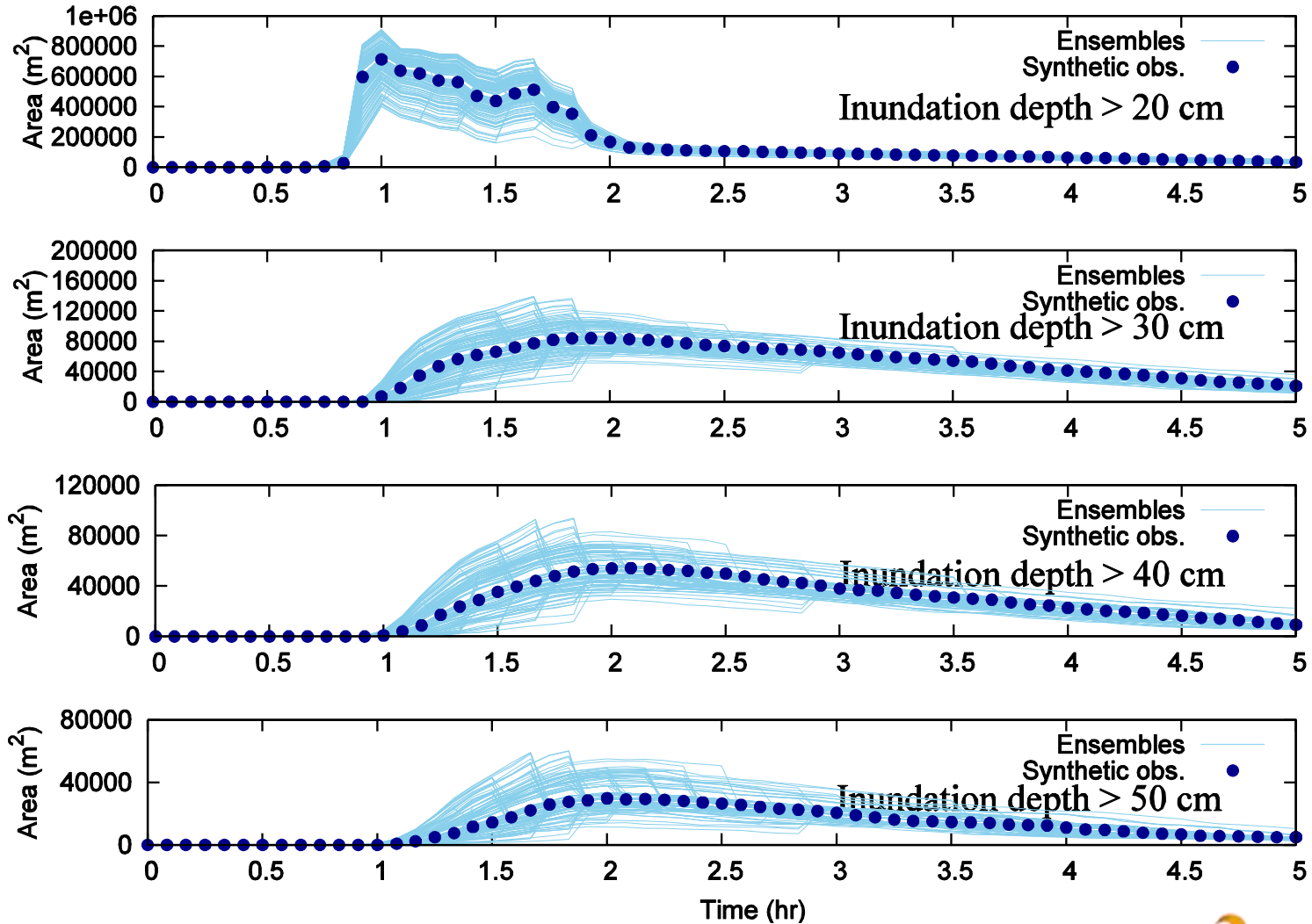
Inundation area – without particle filtering





Application – Comparison on inundation area

Inundation area – with particle filtering

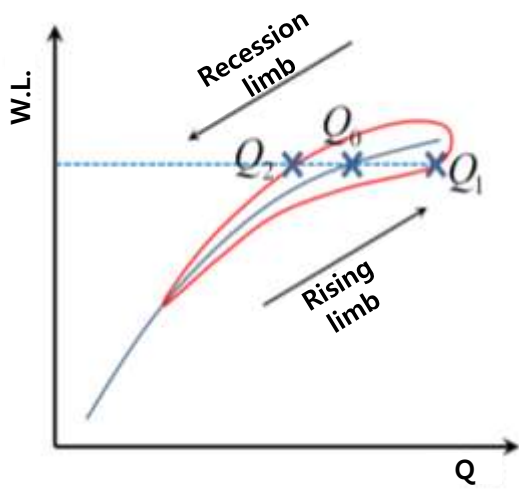
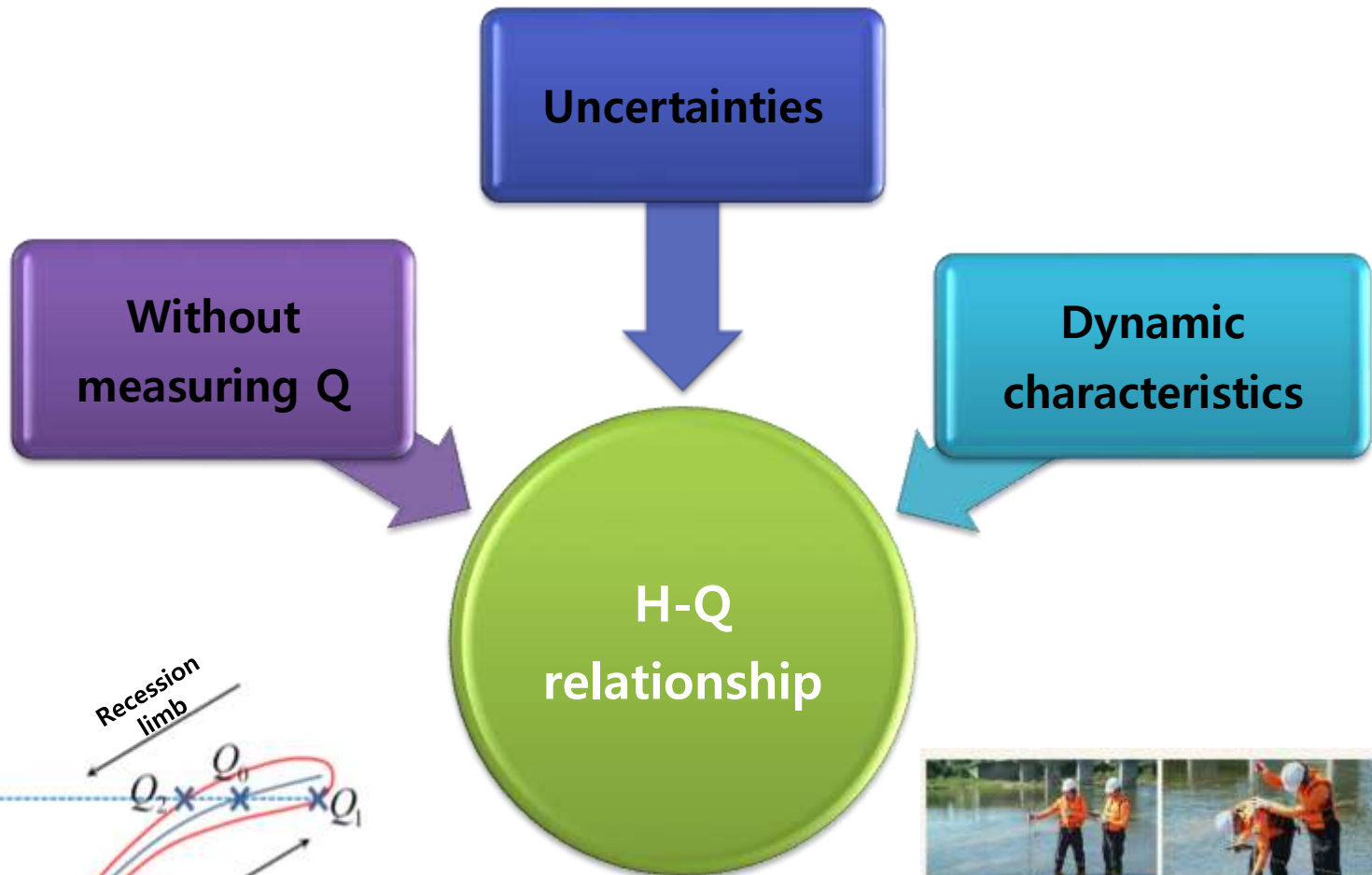




Assimilation-based H-Q Relationships using 2-D Dynamic Wave Model and PF

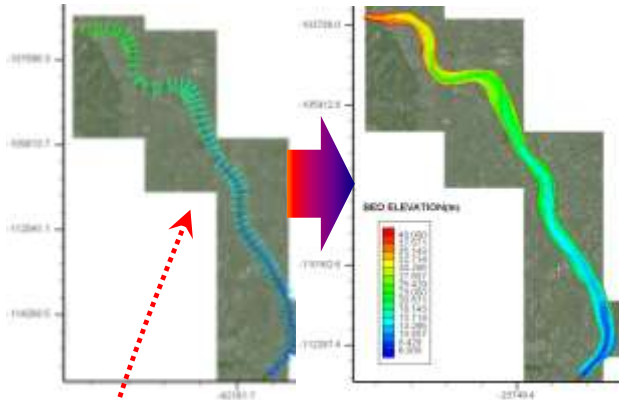


Motivation – Assimilation-based H-Q

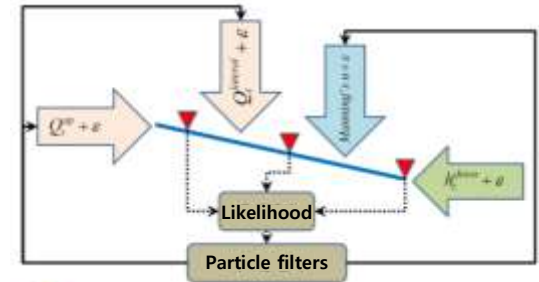


Images from www.hsc.re.kr

Methodology – DA procedures



Setup of 2-D dynamic wave model
(observed data and Aerial photo)

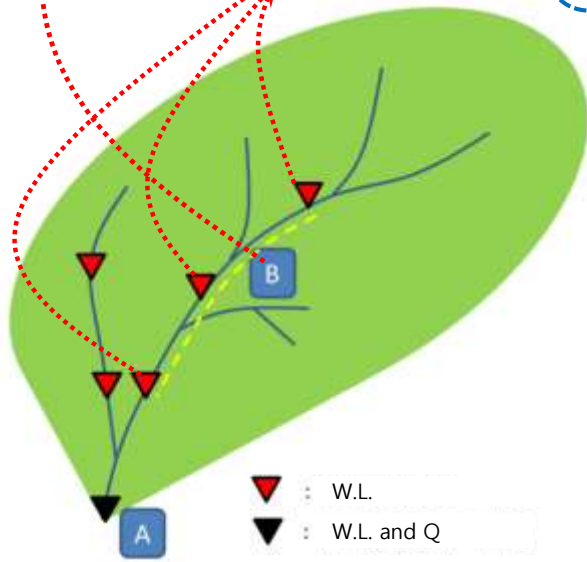


- ▼ Updated W.L.
- Flood routing using a hydraulic model
- Q^u Upstream Q h_c^{down} Lateral Q h_c^{down} Downstream W.L.
- Manning's n ϵ Noise

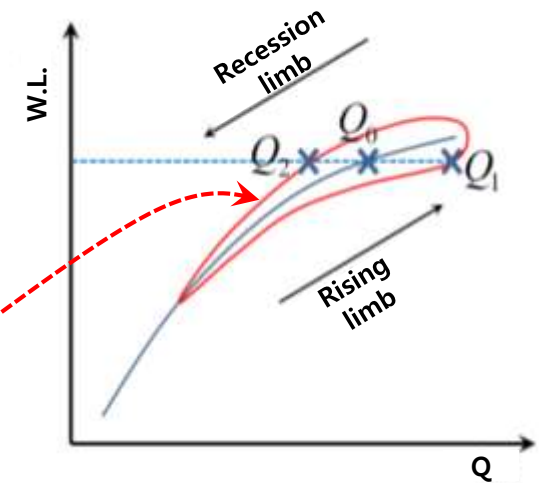
Updating ensembles using observed W.L.

Noise specification
(inflow and roughness)
+
Particle filtering
using observed H

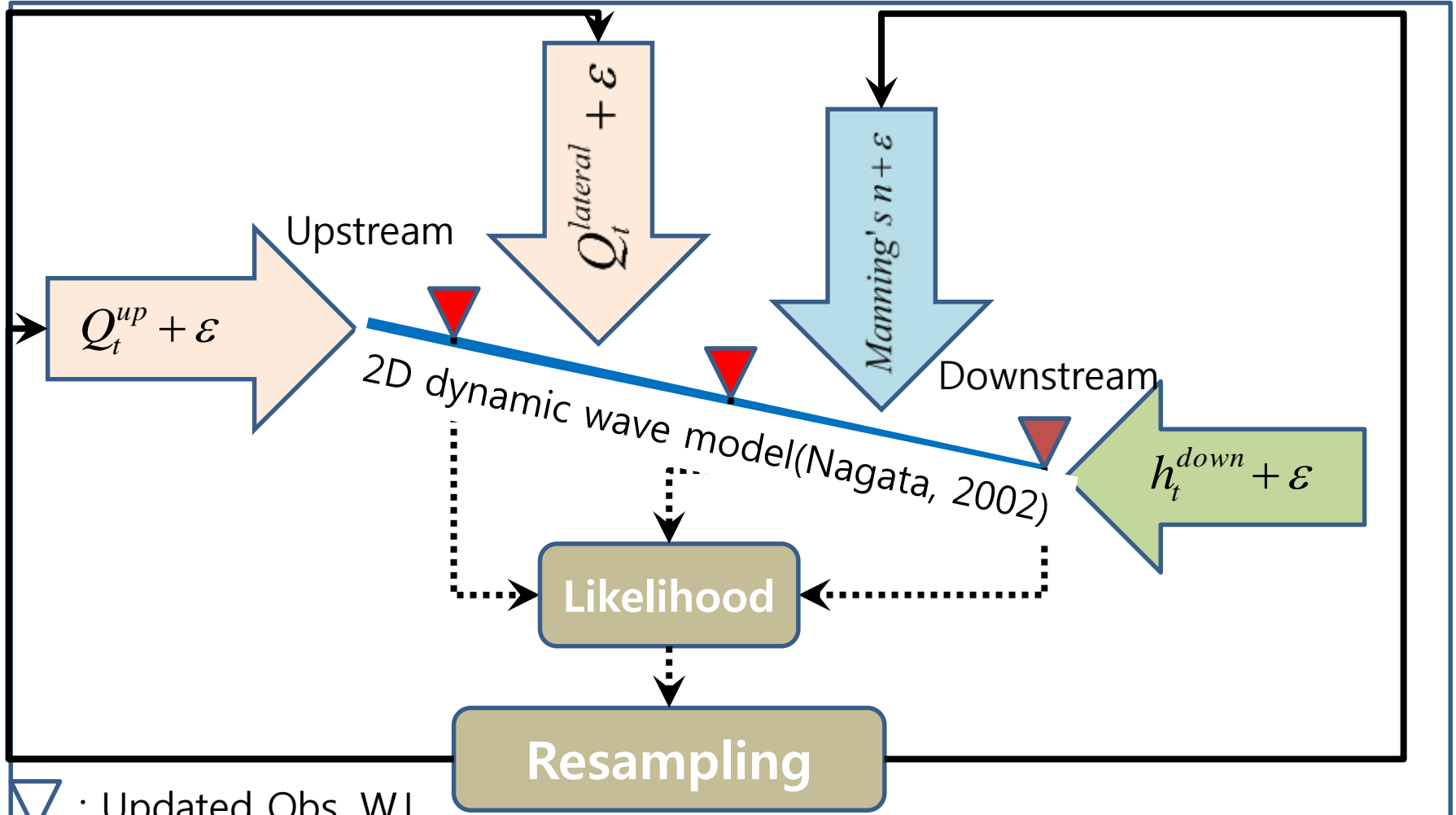
Assimilation-based H-Q



- ▼ : W.L.
- ▼ : W.L. and Q



Methodology – Noise specification & PF



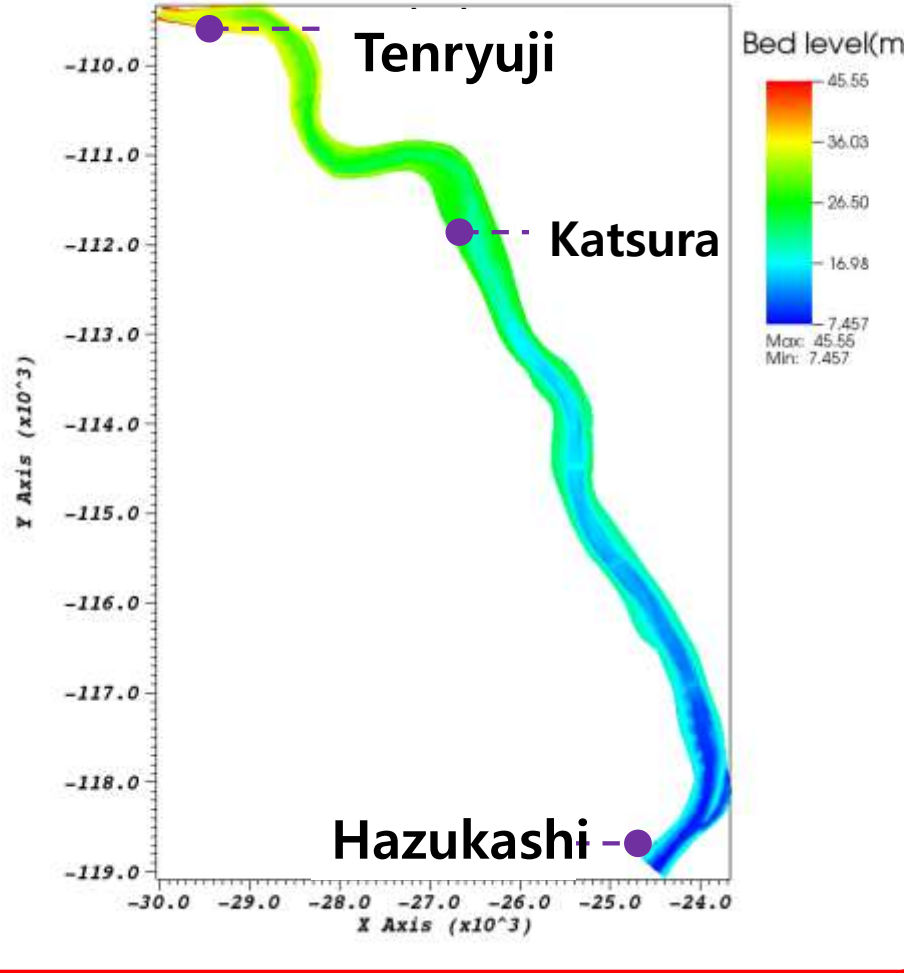
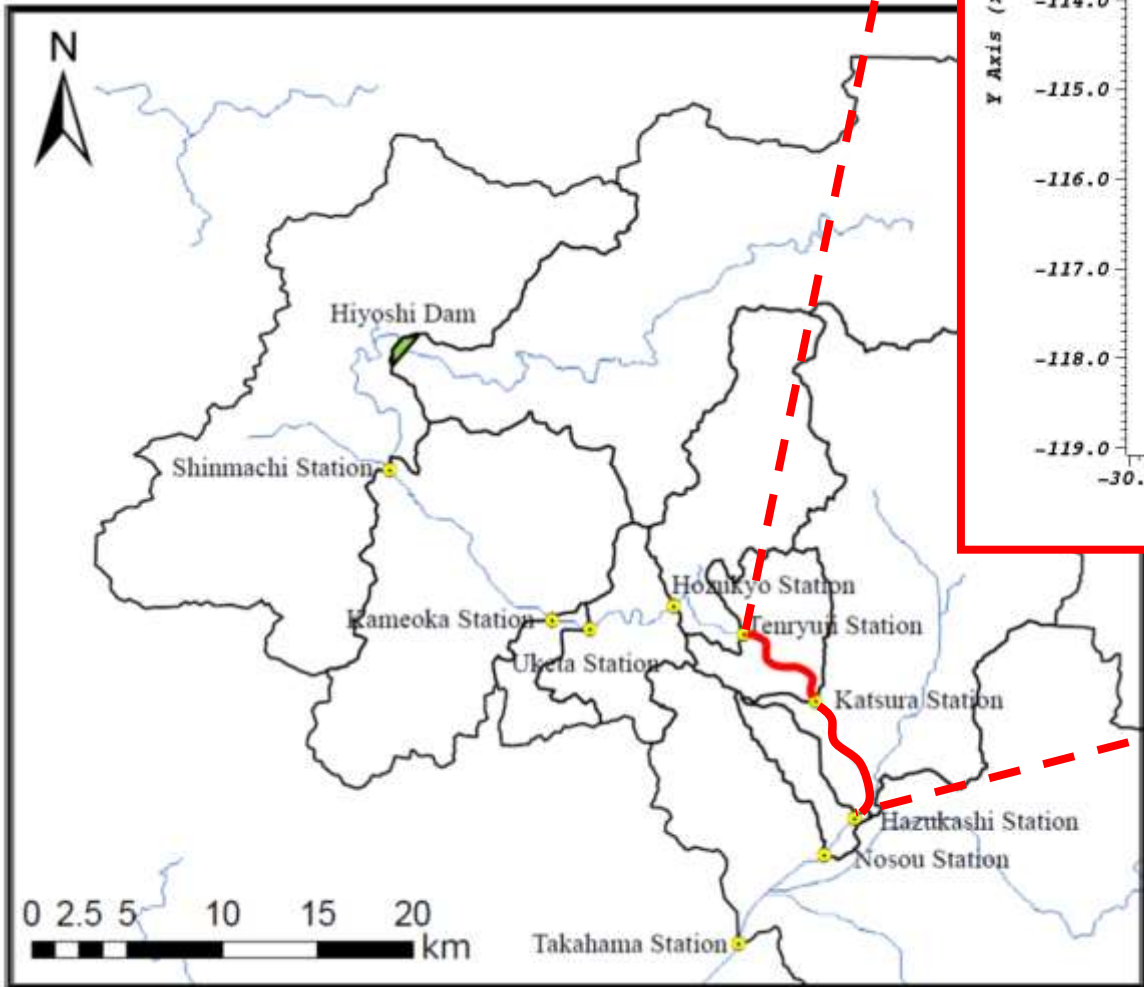
▽ : Updated Obs. W.L.

Q_t^{up} : Upstream Q

$Q_t^{lateral}$: Lateral Q

h_t^{down} : Downstream W.L. ϵ : Noise

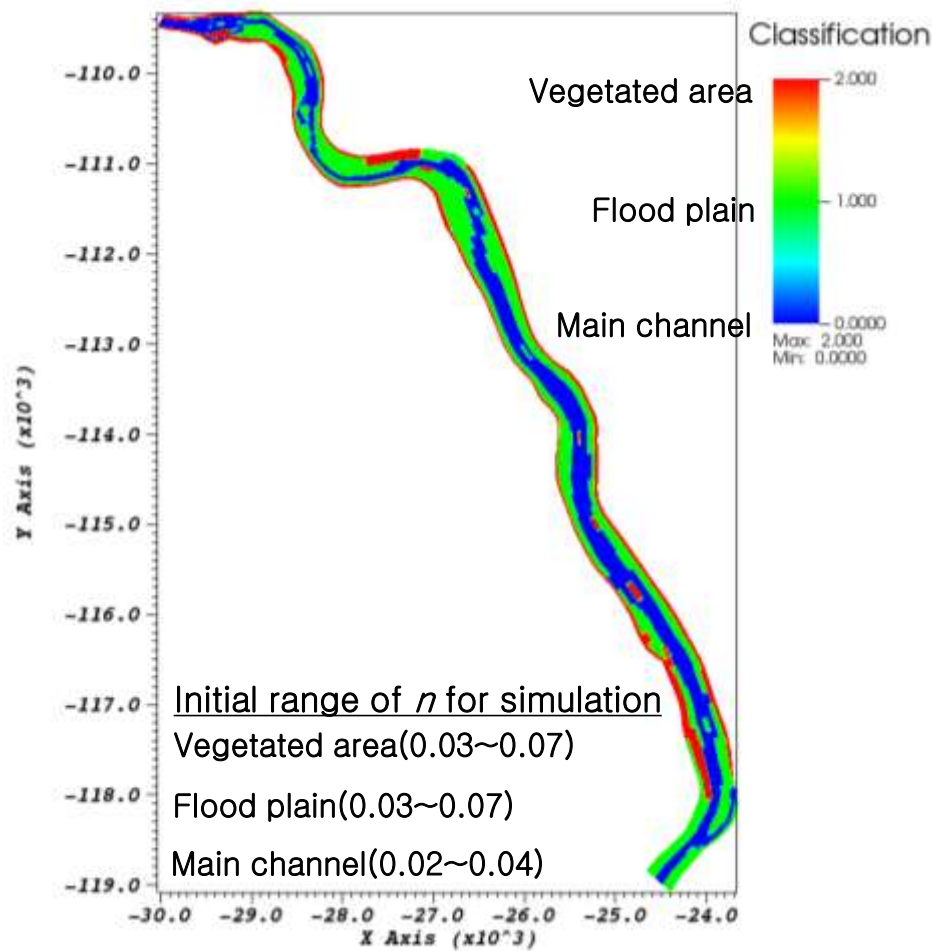
Application – Study area



12.8km)

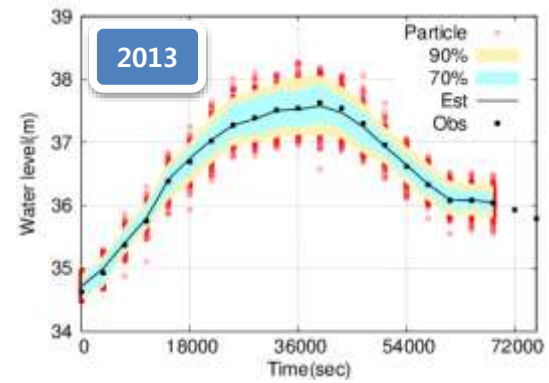
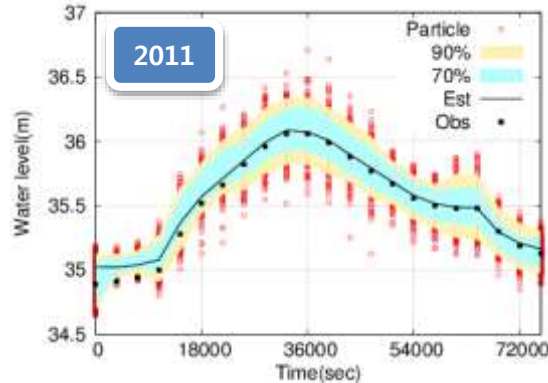
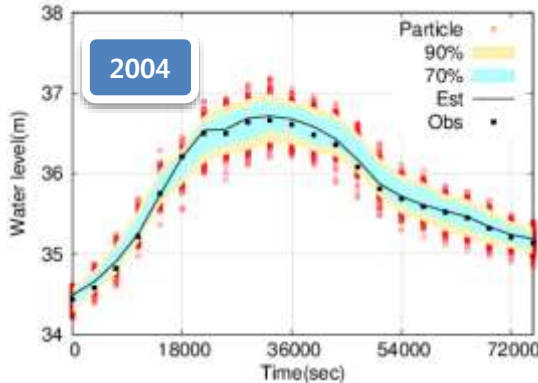
Application - Setup of 2-D dynamic wave model

- Classification of Manning's n

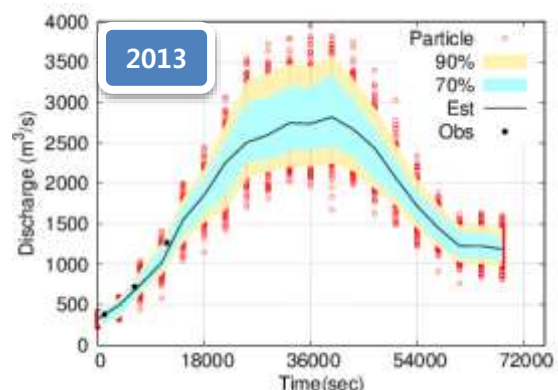
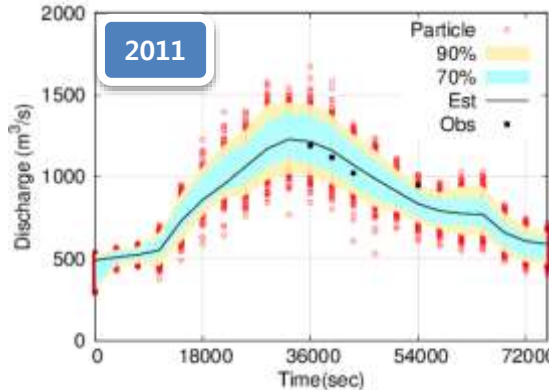
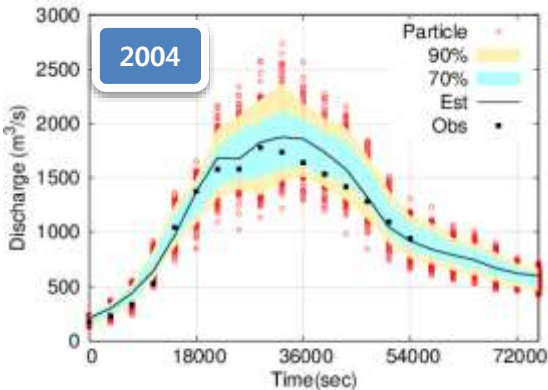


Application – Estimation of H & Q

- Estimated and observed H at Tenryuji station

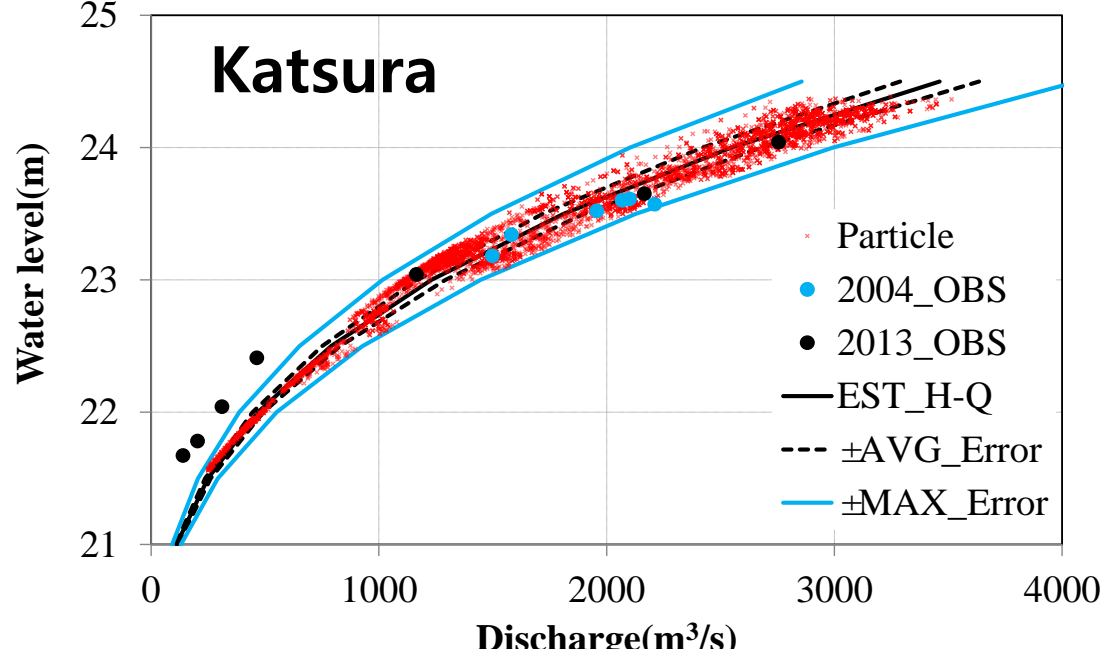
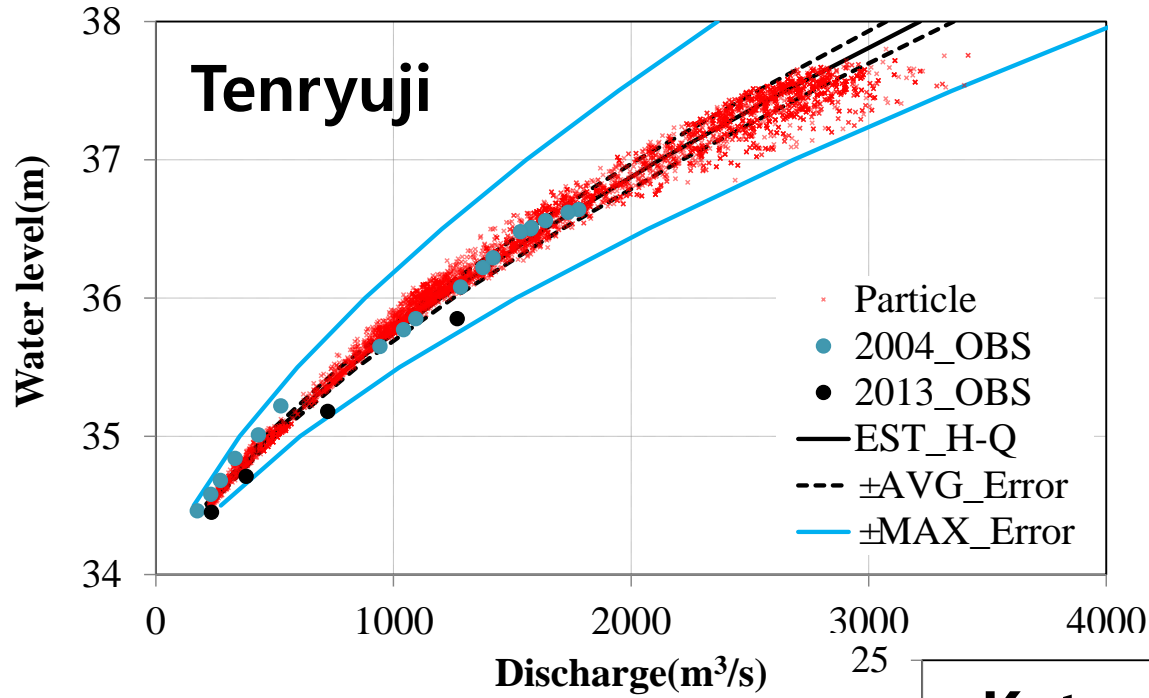


- Estimated and observed Q at Tenryuji station





Application - Assimilation-based H-Q



- **Applicability of particle filtering for two hydraulic models were evaluated.**
- **PF is sound when momentum equilibrium is important in the prediction models**
- **Assimilation-based estimation using 2-D model and PF could provide reliable H-Q relationships for poorly-gauged or ungauged basins**

- PF and dimensionality
 - Different ensembles are required for estimation of states and parameters
 - Hybrid DA to reduce dimensionality
 - PF-MLEF, PF-EnKF, ...
- Real-time applications
 - Parallel computing for ensembles
 - Parallel computing within an ensemble
- More attention is needed to improve DA to consider both covariance and dynamics of the system

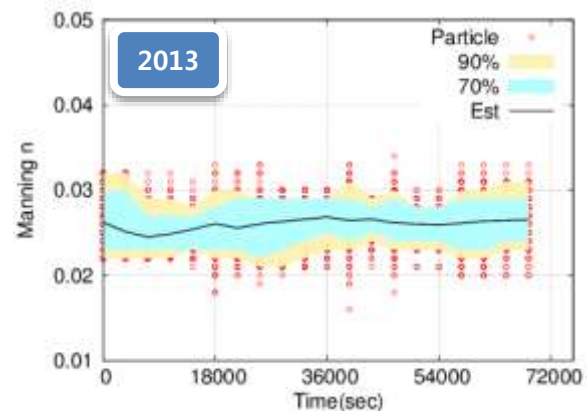
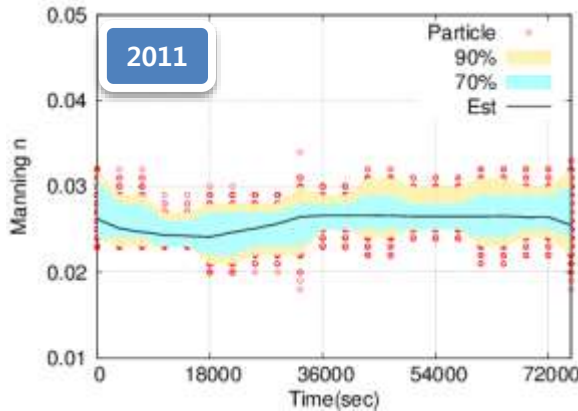
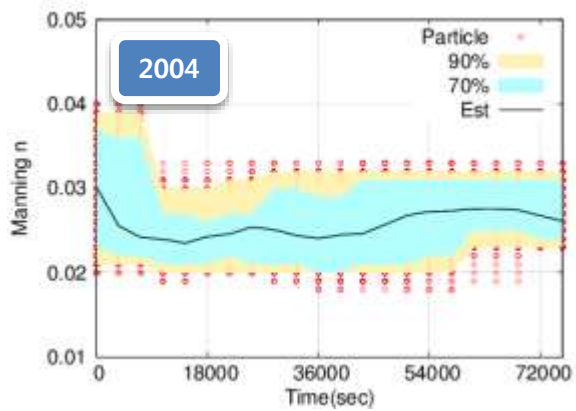


Thank you for your attention!
seongjin.noh@gmail.com

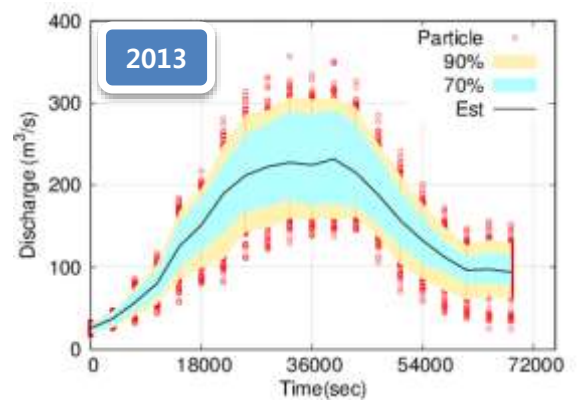
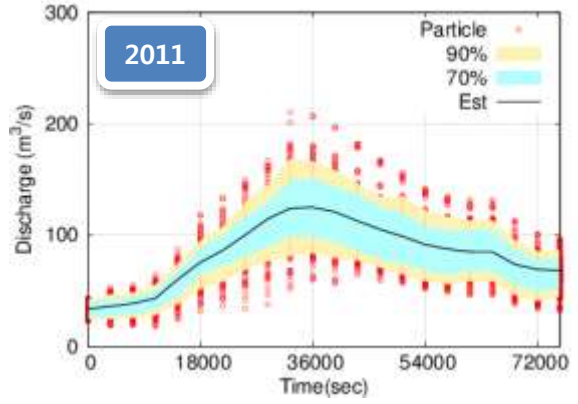
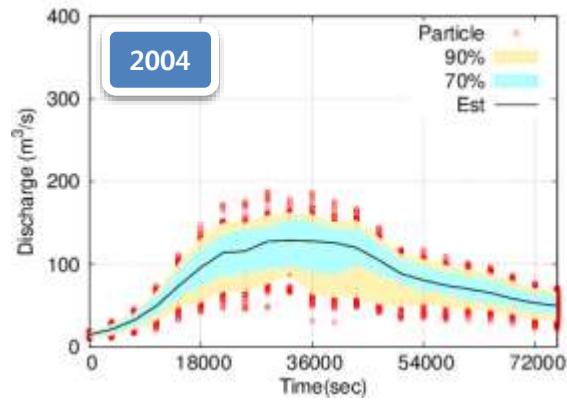


Application – Estimation of n & lateral flow

- Estimated Manning's n at main channel



- Estimated lateral inflow from Tenryuji to Katsura station



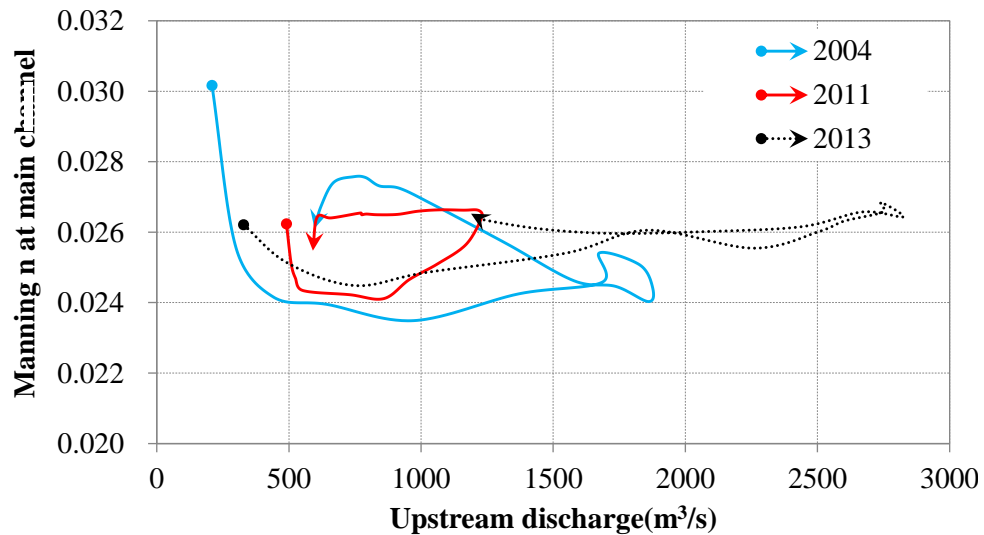
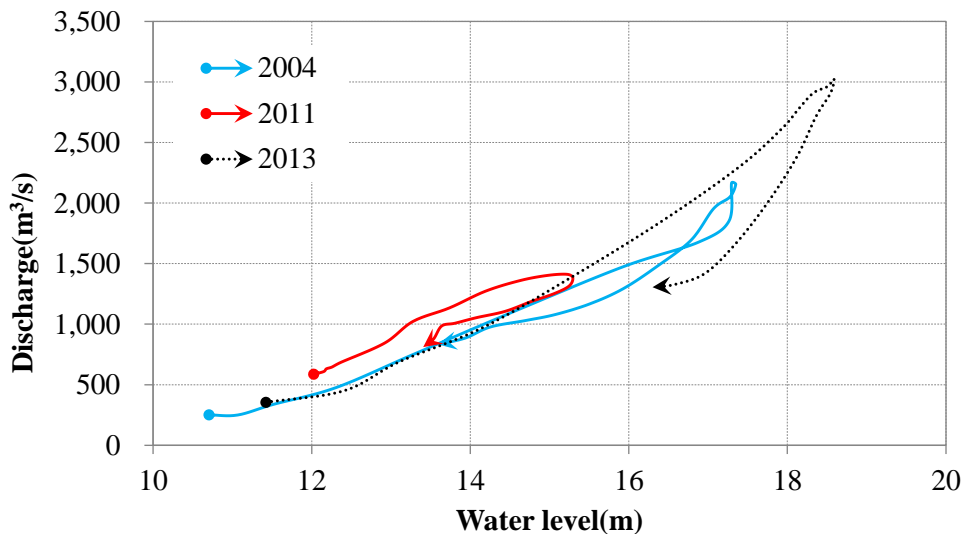
Potential observation for data assimilation



Real-time monitoring system of sewer water level in Seoul, Korea



Dynamic characteristics of H-Q relationship



➤ Connection models

▪ Manhole and sewer pipe

- In the case of $(h_m \geq h_p)$

$$Q = 0.35 \times A_m \times \sqrt{2gh_m} \quad \text{If, } (h_p / h_m > 2/3)$$

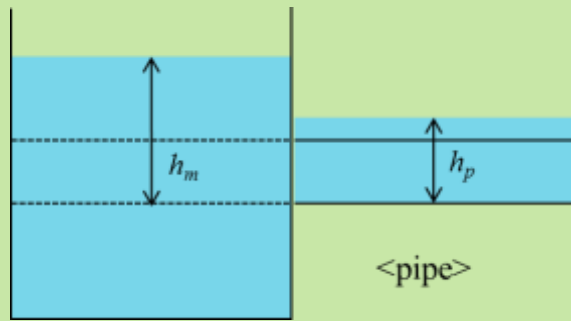
$$Q = 0.91 \times A_p \times \sqrt{2g(h_m - h_p)} \quad \text{If, } (h_p / h_m \leq 2/3)$$

- In the case of $(h_p \geq h_m)$

$$Q = -0.35 \times A_p \times \sqrt{2gh_p} \quad \text{If, } (h_m / h_p > 2/3)$$

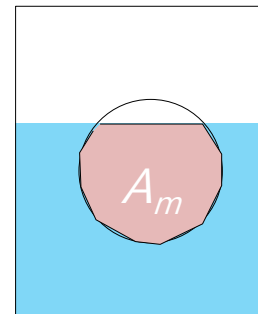
$$Q = -0.91 \times A_m \times \sqrt{2g(h_p - h_m)} \quad \text{If, } (h_m / h_p \leq 2/3)$$

z_m : elevation of manhole z_p : elevation of pipe
 h_m : water depth of manhole h_{cp} : critical depth of pipe
 v_m : velocity of manhole v_p : critical velocity of pipe
 g : gravity acceleration h_p : critical depth of pipe + velocity head



<manhole>

<pipe>



A_m : calculated by h_m
 A_p : calculated by h_{cp}

Fig. Concept of virtual area of manhole

- **Urban inundation** due to heavy rainfall and climate change is an inevitable problem for many cities around the world and constitutes a severe threat to residential life, property and infrastructure(Mark et al., 2004)
- Therefore, it is important **to accurately simulate urban hydrological processes** and efficiently predict the potential risks of urban floods (Lee et al., 2009)
- However, it is **insufficient to obtain accurate predictions due to various uncertainties coming from input forcing data, model parameters, and observations.**

